
Site Investigation and Condition Assessment Report

Reservoir 1
40 Fort Hill Road
Devonshire

Mason and Associates Ltd.
January 14, 2026

Subject Property



Descriptive "North" Indicated here

Inspector Information:

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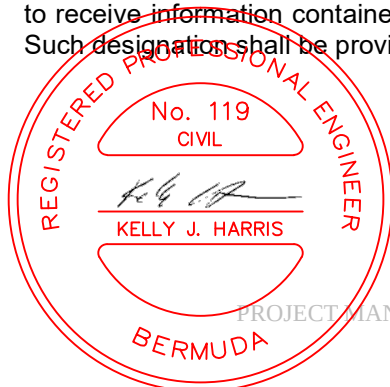
Client Information:

*Ministry of Public Works
32 Palmetto Road,
Devonshire
Attn: Mr. Keith Claridge*

*Phone: (441) 246-8520
Email: kclaridge@gov.bm*

IMPORTANT INFORMATION

This report is solely for the benefit of the above mentioned client. Any person or party designated by the Client to receive information contained in this report shall be subject to the Terms and Conditions contained herein. Such designation shall be provided in writing to the inspector.



PROJECT MANAGERS - STRUCTURAL ENGINEERS - HOME INSPECTORS

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SUMMARY

Reservoir 1 at Fort Prospect, a one-million-gallon capacity tank, is currently out of service due to a significant leak. An April 2025 assessment confirmed cracks at the eastern wall base. A pre-design investigation was commissioned by the Ministry of Public Works to confirm the structure's condition and the conditions behind the walls and floors.

Key Findings:

- **Structure and Ground Conditions:** The concrete thickness of the reservoir's walls (5 to 14 ½ inches) and floor (5 to 11 inches) varied, due to the deviations in the original moat walls and floor. No reinforcing steel was found in the concrete cores extracted from the walls and floors. Generally, rock was found behind the walls and floors.
- **Voids and Cracks:** Localized voids were found under the floor in Section 1 and behind the south wall in Section 3. In Section 4, large cracks at the wall bases are believed to be caused by the settlement of the structure following compaction of the fill material under the floor.
- **Roof/Beams:** Precast beams and slabs were found to be in good condition, requiring no remedial work. However, two areas of spalled concrete and corroded reinforcing steel were observed on an older, reinforced concrete beam at the overflow into Reservoir 2. Numerous leaks through the roof slabs were noted, causing staining and deposits on the beams.

Recommendations:

The following recommendations are provided for the final remediation design:

- **Void Remediation:** The localized voids in Section 1 and Section 3 should be filled with a cement slurry or grout.
- **Reservoir Design:** The new reservoir floors should be designed to span over small voids or soft material, ensuring the slab can bridge the filled area in Section 4.
- **Roof Maintenance:** The roof slab should be resealed to prevent further water ingress, which will mitigate any potential long-term deterioration of the beams from water exposure.

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1.0 Background

Reservoir 1 is located at Fort Prospect in Devonshire and is one of three, one million gallon capacity reservoirs at the site. The reservoir is currently out of service due to a significant leak. An assessment was completed in April 2025 and confirmed that there were cracks at the eastern end of the reservoir. It was decided that the reservoir will remain out of service until full remediation work is completed.

In advance of the detailed remedial design, a pre-design investigation was commissioned by the Ministry of Public Works to confirm the condition of the structure and primarily the ground conditions below the reservoir. The scope included;

- Drilling cores in the floors and walls to investigate the conditions behind the concrete and to confirm the thickness of concrete. The locations were provided by the Client.
- Structural assessment of the existing roof beams and slabs.
- Removal of loose surfaces to examine the concrete walls behind.

The investigation was conducted between the November 17, and December 5, 2025.

2.0 Observations

The reservoir was divided into sections as indicated in Figure 1. Cores and beams were individually numbered and locations are shown on the drawings in Appendix A.

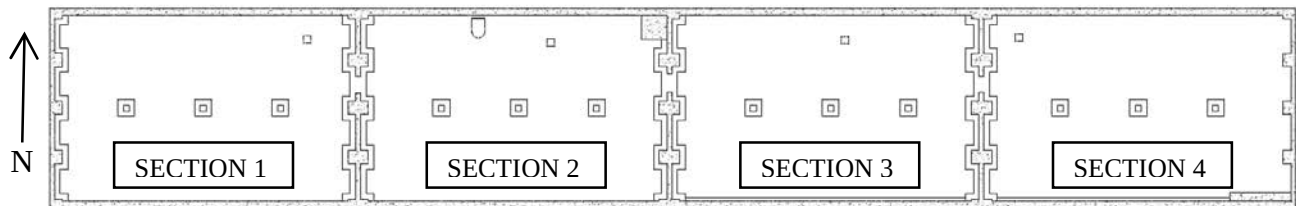


Figure 1. Reservoir 1 Layout

2.1 Floor and Wall Investigation

Cores were drilled using a Vevor Model Z1Z-9260 diamond core drilling machine. They were drilled to a depth of 15 inches, the limit of the machine, and the cores extracted where possible. The thickness of the concrete cores were measured and recorded in Appendix B.

Floor thicknesses varied between 5 inches to 11 inches and wall thicknesses measured between 5 inches and 14 ½ inches. The average floor and wall thicknesses for each section are listed in Tables 1 and 2. No reinforcing steel was encountered in any of the cores.

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Section	Location	Average Thickness (inches)
1	Floor	6.31
2	Floor	7.10
3	Floor	6.79
4	Floor	7.30

Table 1. Average Floor Thickness

Section	Location	Average Thickness (inches)
1	Wall	6.63
2	Wall	7.26
3	Wall	8.03
4	Wall	8.43

Table 2. Average Wall Thickness

A small void was discovered under the north side of the floor in Section 1. An additional core was drilled approximately 3 feet away to determine the extent of the void. It was confirmed that the void was localized. A larger void was found behind the south wall in Section 3. The full extent of the void could not be determined and the depth measurements varied up to several feet. Photos and video footage taken within the irregularly shaped void did not provide additional information about the size.

No issues were observed when the loose coatings were removed from the walls and floors. The concrete below appeared sound.

The initial scope of work included more cores in Section 4 due to the presence of large cracks at the base of the north and south walls. No voids were found behind the walls or under the floor, however, the closest core to the south crack (Core #78) appeared to have fill material below the slab.

The south crack in Section 4 was excavated to determine if there was a void behind. Approximately 6 inches was removed and no void was uncovered. The crack extended further into the wall.

2.2 Beams and Roof Slabs

The precast beams and slabs were inspected from scaffolding erected inside of the reservoir. The beams measured 30 inches deep and 17 inches wide with a notch at the south end.

The beams and slabs were in good condition with no cracks or defects observed. No issues were observed at the beam bearings.

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A Proceq Profometer Rebar Locator was used to check the depth of reinforcing steel in the beam. The depth was approximately 1 – 2 inches. Light soundings of the beams did not reveal any areas of concrete delamination.

At the eastern end of the reservoir, an older reinforced concrete beam was positioned over the overflows into Reservoir 2. Two areas of spalled concrete and corroded reinforcing steel were observed. The beam appeared to be part of the original reservoir construction.

We noted that there were numerous leaks through the roof slabs. This has left staining and deposits on the beams but the leaks did not appear to cause any detrimental effects to the structure.

Roots were visible on various areas on the upper north and south sides of the reservoir. Roots were observed around the access on the south side of Section 3.

Photos of the beams and slabs are included in Section 5.0.

3.0 Conclusions

The concrete thickness of the walls and floor varied. This is likely due to the variation of the original moat walls where the reservoir was built. Generally rock was found behind the walls and floors.

Voids were found in the floor of Section 1 and the south wall of Section 3.

No void was found in Section 4 where the large cracks were located. There appeared to be fill material under one section of the floor and we believe that settlement of the structure over the fill has caused the cracks at the base of the north and south walls. The filled area did not appear to extend far based on the core samples. No reinforcing steel was found in any of the cores confirming that the walls and floors are not reinforced. This is not unusual given the age of the reservoir.

The beams and slabs were all found to be in good condition with no remedial work required.

4.0 Recommendations

As part of the final design of remediation works, we would recommend the following;

- The voids in Section 1 and 3 appear to be localized and we would recommend filling those areas with a cement slurry/grout.
- The new reservoir floors should be designed to span over small voids or soft material. This will ensure that the slab can span over the filled area in Section 4.
- The roof slab should be resealed to prevent water ingress. This will mitigate potential long term deterioration.

5.0 Photos

5.1 Section 1 Beams



Photo 5.1.1. North bearing between B1 and B2.
Roots visible.



Photo 5.1.2. B1 looking south.



Photo 5.1.3. B1 looking north.



Photo 5.1.4. B1 north bearing.

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Photo 5.1.5. B1 looking toward south bearing.



Photo 5.1.6. Beam B2.



Photo 5.1.7. B2 north bearing.



Photo 5.1.8. Beam B2.



Photo 5.1.9. Slab between B1 and B2.



Photo 5.1.10. Rebar scan of B1.

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Photo 5.1.11. Rebar scan of B1.



Photo 5.1.12. Rebar scan of B1.



Photo 5.1.13. North bearing area between B2 and B3.



Photo 5.1.14. North bearing B2. Small roots observed.



Photo 5.1.15. Beam B2.



Photo 5.1.16. Beam B3 north end.

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Photo 5.1.17. Beam B3.



Photo 5.1.18. Underside of slabs between B2 and B3.



Photo 5.1.19. Typical water ingress between slabs over B2.



Photo 5.1.20. North bearing of B3. Roots visible.



Photo 5.1.21. Water ingress over B3.



Photo 5.1.22. Underside of slab between B3 and B4.

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Photo 5.1.23. Beam B3.



Photo 5.1.24. North Bearing area between B3 and B4. Roots observed growing into concrete block.



Photo 5.1.25. Beam B4 north bearing.



Photo 5.1.26. Beam B4.



Photo 5.1.27. Old repair under B4.



Photo 5.1.28. Beam B4 north bearing. Roots visible.

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Photo 5.1.29. Beam B4.



Photo 5.1.30. Underside of slabs between B4 and B5.



Photo 5.1.31. Beam B5.



Photo 5.1.32. Underside of B5.



Photo 5.1.33. South bearing area between B4 and B5.



Photo 5.1.34. Beam B5.

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Photo 5.1.35. Underside of slab between B5 and B6.



Photo 5.1.36. Beam B5 looking north.



Photo 5.1.37. South bearing area between B3 and B4. Roots observed.



Photo 5.1.38. South end of Beam B4.

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5.2 Section 2 Beams



Photo 5.2.1. South end of B6 bearing.



Photo 5.2.2. Water ingress and staining B6.



Photo 5.2.3. Beam B6.



Photo 5.2.4. Underside of slab between B6 and B7.



Photo 5.2.5. Water ingress between slabs.



Photo 5.2.6. North Bearing B6.

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Photo 5.2.7. Beam B6 looking south.



Photo 5.2.8. B7 south bearing.



Photo 5.2.9. B7 water ingress visible.



Photo 5.2.10. B7 with water ingress staining.



Photo 5.2.11. B7 looking north.



Photo 5.2.12. Water and staining visible under slab.

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Photo 5.2.13. Underside of slab between B7 and B8.



Photo 5.2.14. B7 south bearing.



Photo 5.2.15. B8 south bearing.



Photo 5.2.16. Beam B8 with staining visible from water ingress.



Photo 5.2.17. B8 south bearing.



Photo 5.2.18. B8 south bearing.

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Photo 5.2.19. B8 south bearing.



Photo 5.2.20. B8 looking north.



Photo 5.2.21. B9 with staining visible.



Photo 5.2.22. B9 with staining from water ingress.



Photo 5.2.23. B9 south bearing.



Photo 5.2.24. B9 south bearing.

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Photo 5.2.25. Beam B9.



Photo 5.2.26. Underside of slab between B9 and B10.



Photo 5.2.27. B10 south bearing.



Photo 5.2.28. B10 looking north.



Photo 5.2.29. B10 south bearing.



Photo 5.2.30. B10 looking north.

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Photo 5.2.31. Water filled tower at divide wall near B10.



Photo 5.2.32. B10 north bearing.



Photo 5.2.33. B10 plastic on the underside of the beam likely from the initial installation. No structural issue.



Photo 5.2.34. B9 north bearing.



Photo 5.2.35. North bearing area between B9 and B10.



Photo 5.2.36. B9 with water staining visible.

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Photo 5.2.37. North bearing area between B8 and B9. Roots visible.



Photo 5.2.38. Deposits and staining on north side of B9.



Photo 5.2.39. B9 north bearing.



Photo 5.2.40. B8 north bearing.



Photo 5.2.41. Root growth through slab joints between B8 and B9.



Photo 5.2.42. B8 staining and water ingress.

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Photo 5.2.43. B8 north bearing.



Photo 5.2.44. B7 north bearing.



Photo 5.2.45. B7 with water staining.



Photo 5.2.46. B6 north bearing.



Photo 5.2.47. Deposits from water ingress between B5 and B6.



Photo 5.2.48. North bearing area between B5 and B6.

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Photo 5.2.49. B5 north bearing.



Photo 5.2.50. B5 looking south.

5.3 Section 3 Beams



Photo 5.3.1. North bearing between B11 and B12.
Roots observed.



Photo 5.3.2. North bearing B11.



Photo 5.3.3. Beam B11 looking south.



Photo 5.3.4. B11 looking north - west side.

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Photo 5.3.5. B11 looking west.



Photo 5.3.6. Slabs between B11 and B12.



Photo 5.3.7. North bearing B12.



Photo 5.3.8. Beam B12.



Photo 5.3.9. North bearing area between B12 and B13.



Photo 5.3.10. B12 north bearing.

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Photo 5.3.11. Beam B12.



Photo 5.3.12. Beam B12.



Photo 5.3.13. B13 north bearing.



Photo 5.3.14. Beam B13.



Photo 5.3.15. Slabs between B12 and B13.



Photo 5.3.16. North bearing area between B13 and B14.

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Photo 5.3.17. Beam B13 north bearing. Roots visible.



Photo 5.3.18. Beam B13.



Photo 5.3.19. Slabs between B13 and B14.



Photo 5.3.20. B14 bearing.



Photo 5.3.21. Beam B14.



Photo 5.3.22. B14 north bearing east side.

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Photo 5.3.23. North bearing area between B14 and B15. Tree roots observed.



Photo 5.3.24. Beam B14.



Photo 5.3.25. Beam B14.



Photo 5.3.26. B15 north bearing.



Photo 5.3.27. Beam B15.



Photo 5.3.28. Slabs between B14 and B15.

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Photo 5.3.29. Beam B15.



Photo 5.3.30. Beam B11 south bearing.



Photo 5.3.31. Roots growing in manhole.



Photo 5.3.32. B11 south bearing.



Photo 5.3.33. South bearing area between B11 and B12.



Photo 5.3.34. Beam B12.

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Photo 5.3.35. B12 south bearing.



Photo 5.3.36. Beam B12.



Photo 5.3.37. South bearing area between B12 and B13.



Photo 5.3.38. B13 south bearing.



Photo 5.3.39. Beam B13.



Photo 5.3.40. South bearing B13.

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Photo 5.3.41. Beam B13.



Photo 5.3.42. Beam B13 with staining visible.



Photo 5.3.43. Opening created in the slab between B13 and B14.



Photo 5.3.44. South bearing area between B13 and B14.



Photo 5.3.45. Minor damage to concrete block between B13 and B14.



Photo 5.3.46. South bearing B14.

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Photo 5.3.47. Beam B14.



Photo 5.3.48. South bearing B14.



Photo 5.3.49. South bearing area between B14 and B15.



Photo 5.3.50. South bearing B15.



Photo 5.3.51. Beam B15 with active leaks visible.



Photo 5.3.52. Underside of B15.

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5.4 Section 4 Beams



Photo 5.4.1. B16 north bearing.



Photo 5.4.2. Underside of B16.



Photo 5.4.3 Beam B16.



Photo 5.4.4 North bearing B16.

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Photo 5.4.5 North bearing area between B16 and B17.



Photo 5.4.6 North bearing B17.



Photo 5.4.7 Beam B17.



Photo 5.4.8 Underside of slabs between B16 and B17.



Photo 5.4.9 North bearing area between B17 and B18.



Photo 5.4.10 North bearing B17.

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Photo 5.4.11 Staining on B17.



Photo 5.4.12 Beam B17.



Photo 5.4.13 Underside of slabs between B17 and B18.



Photo 5.4.14 North bearing B18.



Photo 5.4.15 Underside of Beam B18.



Photo 5.4.16 Beam B18.

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Photo 5.4.17 North bearing area between B18 and B19.



Photo 5.4.18 North bearing B18.



Photo 5.4.19 Beam B19.



Photo 5.4.20 Beam B19.



Photo 5.4.21 Underside of slabs between B18 and B19.



Photo 5.4.22 North bearing B19.

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Photo 5.4.23 Beam B19.



Photo 5.4.24 North bearing between B19 and B20.



Photo 5.4.25 North bearing B19.



Photo 5.4.26 Beam B19.



Photo 5.4.27 North bearing B20.



Photo 5.4.28 Beam B20.

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Photo 5.4.29 South bearing area between B19 and B20.



Photo 5.4.30 South bearing B20.



Photo 5.4.31 South bearing B19.



Photo 5.4.32 Corroded reinforcing steel at the eastern edge beam.



Photo 5.4.33 Corroded reinforcing was observed.



Photo 5.4.34 Corroded reinforcing at eastern edge beam.

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Photo 5.4.35 Corroded stirrups.



Photo 5.4.36 South bearing B19.



Photo 5.4.37 South bearing area between B18 and B19.



Photo 5.4.38 South bearing B18.



Photo 5.4.39 South bearing B18.



Photo 5.4.40 South bearing area between B17 and B18.

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Photo 5.4.41 South bearing B18.



Photo 5.4.42 South bearing B17.



Photo 5.4.43 Staining on B17.



Photo 5.4.44 South bearing area between B16 and B17.



Photo 5.4.45 South bearing B17.



Photo 5.4.46 South bearing B16.

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Photo 5.4.47 South bearing area between B15 and B16.



Photo 5.4.48 South bearing B16.

5.5 Coating Removal



Photo 5.5.1 Hollow section in Section 3 Floor (eastern end).



Photo 5.5.2 North wall section 3.

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Photo 5.5.3 North wall section 3.



Photo 5.5.4 North wall section 3.



Photo 5.5.5 North wall section 2.



Photo 5.5.6 North wall section 2.



Photo 5.5.7 North wall section 2.



Photo 5.5.8 North wall section 2.

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Photo 5.5.9 South wall section 2.



Photo 5.5.10 South wall section 2.



Photo 5.5.11 South wall section 2.



Photo 5.5.12 South wall section 2.



Photo 5.5.13 North wall section 1.

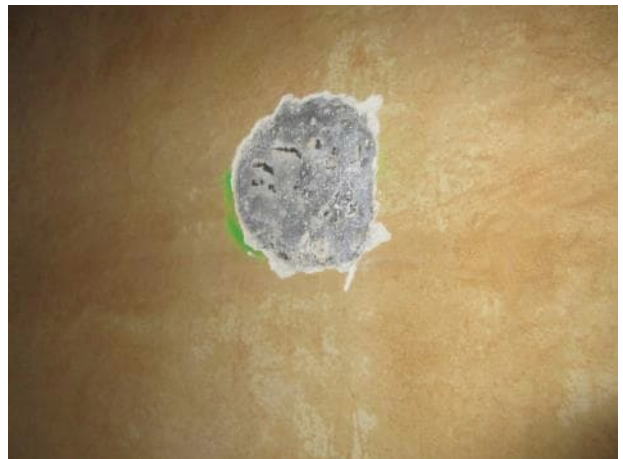


Photo 5.5.14 North wall section 1.

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Photo 5.5.15 North wall Section 4.



Photo 5.5.16 North wall Section 4.



Photo 5.5.17 Hollow floor coating in Section 4 south side.

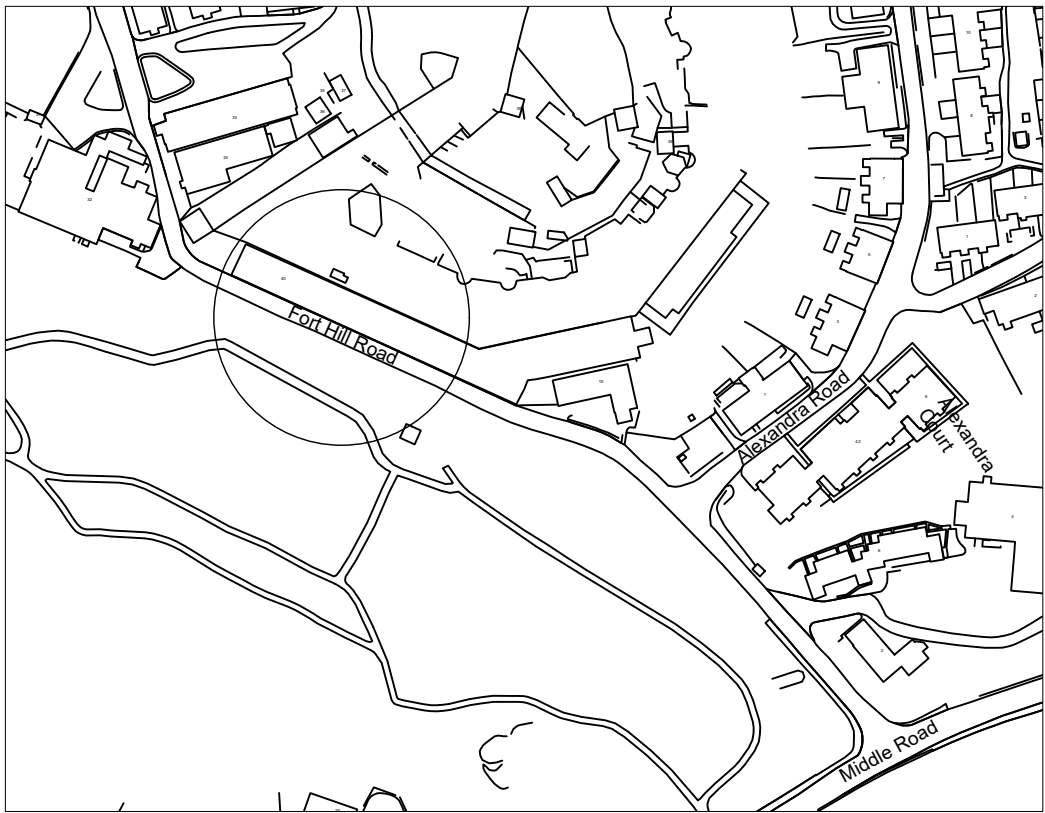


Photo 5.5.18 Exposed crack at base of wall on south side of Section 4.

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APPENDIX A

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LOCATION PLAN
Scale: 1:2500

P.O. BOX 1477, Hamilton HM FX
Phone: (441) 331-1961
Email: info@mason.bm
www.mason.bm

CLIENT

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INFORMATION

IMPORTANT NOTES

This seal indicates that Mason and Associates Ltd.(MAL) has had direct or indirect input into specific design elements of the works. As such our company must be afforded the opportunity to inspect the work as it progresses to ensure conformity to the related design details, specifications and notes. If we are not accorded the privilege of inspecting the work on site before it is covered up or otherwise hidden or if variations to the original details are made without written approval by the engineer of record, Mason and Associates Ltd. will assume no responsibility for the works. Client is still required to contact Building Control for inspections.

DATE: MAL ENGINEER: COMMENTS INSPECTION:

REVISIONS

No.	DATE	BY	REVISION
A	JUN 2025	KC	COREHOLES PROVISIONAL LOCATION

PROJECT DETAILS

AS-BUILT

RESERVOIR 1
10 FORT HILL ROAD
DEVONSHIRE

TITLE

FLOOR AND ROOF PLANS

DRAWN BY: KJH

CHECKED BY: KJH

JOB No: 15483

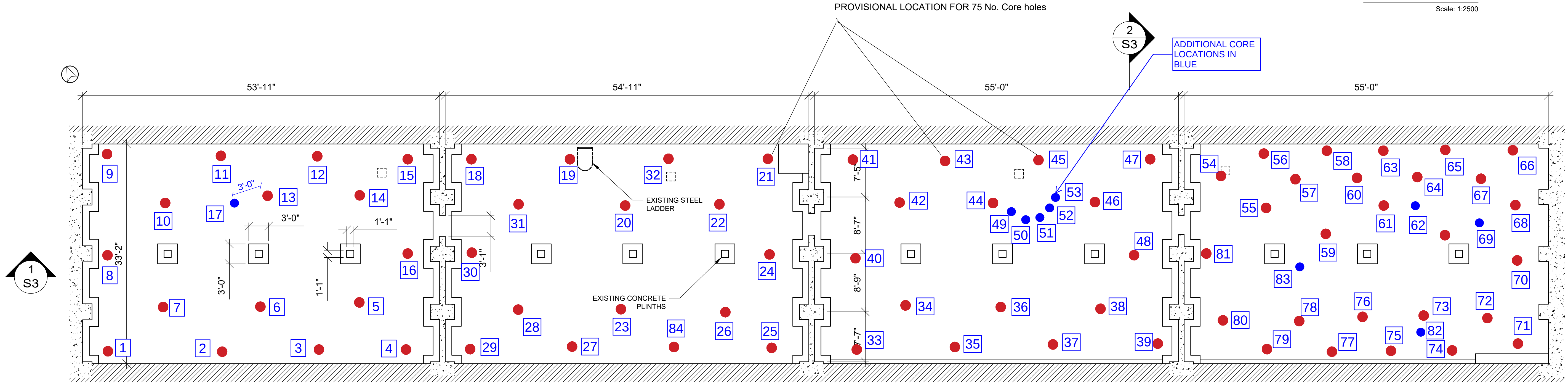
DATE: MARCH 24 2025

REVISION:

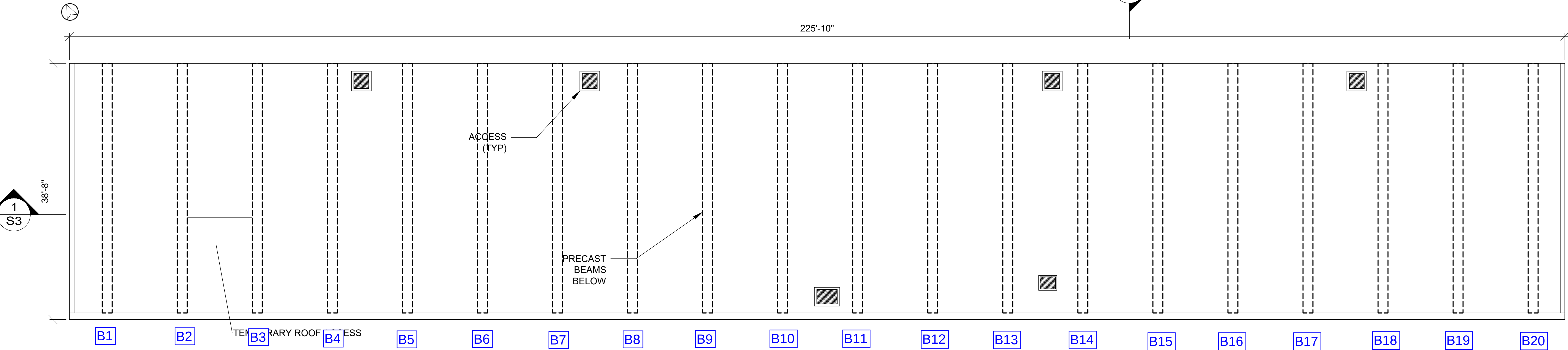
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SHEET No:

S1

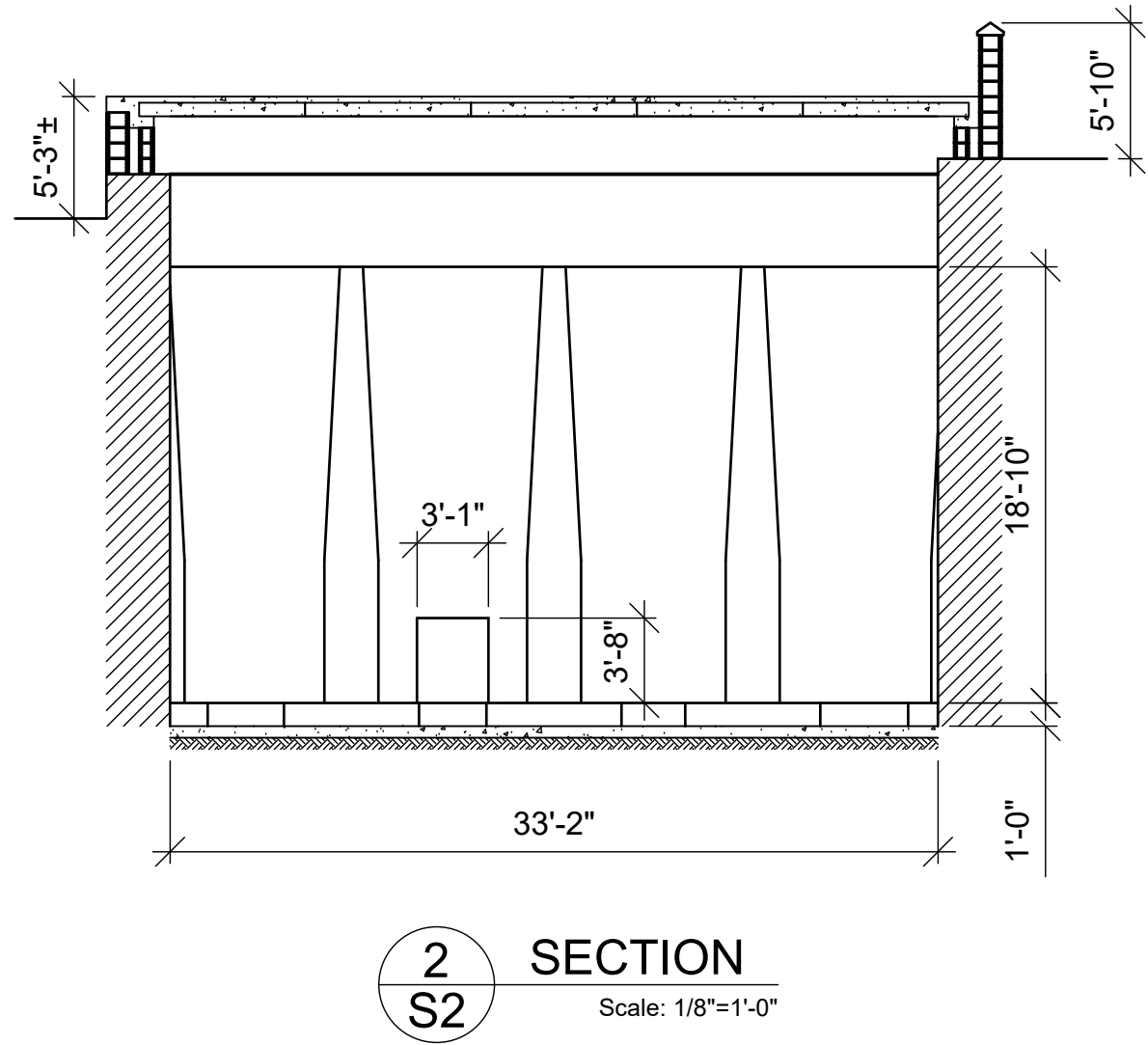
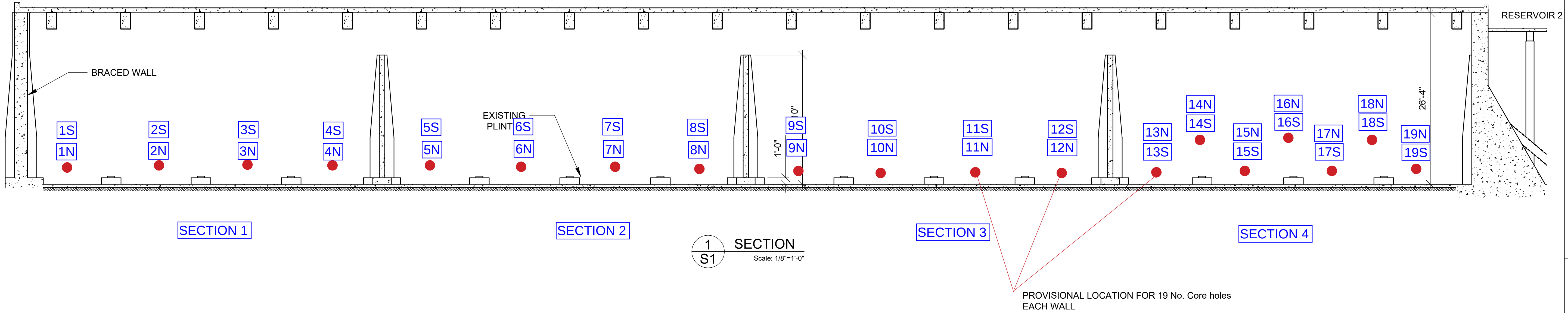


FLOOR PLAN
Scale: 1/8" = 1'-0"



ROOF PLAN
Scale: 1/8" = 1'-0"

CLIENT



IMPORTANT NOTES

This seal indicates that Mason and Associates Ltd.(MAL) has had direct or indirect input into specific design elements of the works. As such our company must be afforded the opportunity to inspect the work as it progresses to ensure conformity to the related design details, specifications and notes. If we are not accorded the privilege of inspecting the work on site before it is covered up or otherwise hidden or if variations to the original details are made without written approval by the engineer of record, Mason and Associates Ltd. will assume no responsibility for the works. Client is still required to contact Building Control for inspections.

DATE MAL ENGINEER COMMENTS INSPECTION

REVISIONS

No.	DATE	BY	REVISION
A	JUN 2025	KC	COREHOLES PROVISIONAL LOCATION

PROJECT DETAILS

AS-BUILT

RESERVOIR 1
10 FORT HILL ROAD
DEVONSHIRE

TITLE

SECTIONS

DRAWN BY: KJH

CHECKED BY: KJH

JOB No: 15483

DATE: MARCH 24 2025

REVISION:

A

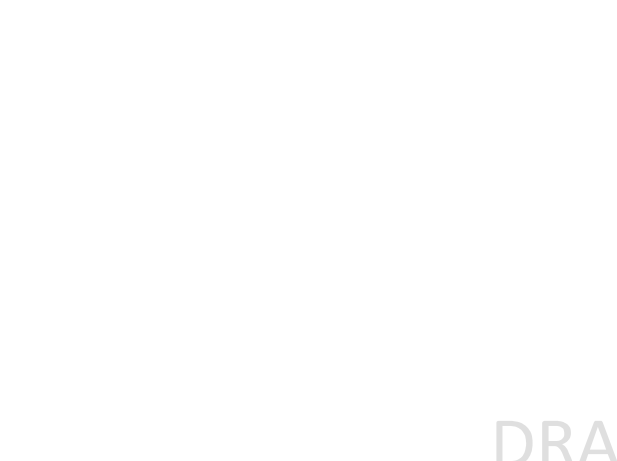
SHEET No:

S2

APPENDIX B

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CONCRETE CORE SPECIMENS

No:	Date	Location	Length	Diameter
1	24 Nov 2025	Section 1; floor	7"	4"
				
2	24 Nov 25	Section 1; floor	5"	4"
				
3	24 Nov 25	Section 1; floor	5"	4"
				

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4	24 Nov 25	Section 1; floor	8"	4"
				
5	24 Nov 25	Section 1; floor	5"	4"
				
6	24 Nov 25	Section 1; floor	5 1/2"	4"
				

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7	25 Nov 25	Section 1; floor	5"	4"
				
8	25 Nov 25	Section 1; floor	5"	4"
				
9	25 Nov 25	Section 1; floor	5 1/2"	4"
				

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10	25 Nov 25	Section 1; floor	5"	4"
				
11	25 Nov 25	Section 1; floor	5 1/4"	4"
				
12	25 Nov 25	Section 1; floor	11"	4"
				

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13	25 Nov 25	Section 1; floor 15" deep with small void below	5"	4"
				
14	25 Nov 25	Section 1; floor	5 1/4"	4"
				
15	25 Nov 25	Section 1; floor	7"	4"
				

16	25 Nov 25	Section 1; floor	8 ½"	4"
				
17	26 Nov 25	Section 1; floor Near void	7"	4"
				
1N	26 Nov 25	Section 1; north wall	6" - 8"	4"
				

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2N	26 Nov 25	Section 1; north wall	9" - 10"	4"
				
3N	26 Nov 25	Section 1; north wall	5"	4"
				
4N	26 Nov 25	Section 1; north wall	6 1/2"	4"
				

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1S	26 Nov 25	Section 1; south wall	6 ½"	4"
				
2S	26 Nov 25	Section 1; south wall	6"	4"
				
3S	26 Nov 25	Section 1; south wall	8"	4"
				

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4S	24 Nov 25	Section 1; south wall	7"	4"
				
18	28 Nov 25	Section 2; floor	7" – 9"	4"
				
19	28 Nov 25	Section 2; floor	6 3/4"	4"
				

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20	28 Nov 25	Section 2; floor	5 ¼"	4"
				
21	28 Nov 25	Section 2; floor	7 ¼"	4"
				
22	28 Nov 25	Section 2; floor	6 ½"	4"
				

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23	28 Nov 25	Section 2; floor	8"	4"
				
24	29 Nov 25	Section 2; floor	6"	4"
				
25	29 Nov 25	Section 2; floor	7 - 8"	4"
				

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26	29 Nov 25	Section 2; floor	7"	4"
				
27	29 Nov 25	Section 2; floor	5 1/2"	4"
				
28	29 Nov 25	Section 2; floor	5 1/2"	4"
				

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29	29 Nov 25	Section 2; floor	11"	4"
				
30	29 Nov 25	Section 2; floor	5"	4"
Core broken				
31	29 Nov 25	Section 2; floor	5"	4"
Core broken				

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32	29 Nov 25	Section 2; floor	5 ½"	4"
				
33	1 Dec 25	Section 3; floor	5 ½"	4"
				
34	1 Dec 25	Section 3; floor	6"	4"
				

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35	1 Dec 25	Section 3; floor	6 ½"	4"
				
36	1 Dec 25	Section 3; floor	5 ½"	4"
				
37	1 Dec 25	Section 3; floor	5 ½"	4"
				

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38	1 Dec 25	Section 3; floor	6"	4"
				
39	1 Dec 25	Section 3; floor	6 1/4"	4"
				
40	1 Dec 25	Section 3; floor	8"	4"
				

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41	1 Dec 25	Section 3; floor	9"	4"
				
42	1 Dec 25	Section 3; floor	6 1/2"	4"
				
43	1 Dec 25	Section 3; floor	6"	4"
				

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44	1 Dec 25	Section 3; floor	5 ½"	4"
				
45	1 Dec 25	Section 3; floor	6"	4"
				
46	1 Dec 25	Section 3; floor	6 ½"	4"
				

DRAFT

47	1 Dec 25	Section 3; floor	6"	4"
				
48	1 Dec 25	Section 3; floor	6 1/2"	4"
				
49	1 Dec 25	Section 3; floor	6"	4"
				

DRAFT

50	1 Dec 25	Section 3; floor	6 ½"	4"
				
51	1 Dec 25	Section 3; floor	6"	4"
				
52	1 Dec 25	Section 3; floor	6 ½"	4"
				

DRAFT

53	1 Dec 25	Section 3; floor	6 ¼"	4"
				
54	1 Dec 25	Section 4; floor	-	4"
Core not retrieved				
55	1 Dec 25	Section 4; floor	6 ¼"	4"
				

DRAFT

56	1 Dec 25	Section 4; floor	6 ¼"	4"
				
57	2 Dec 25	Section 4; floor	7 ½"	4"
				
58	2 Dec 25	Section 4; floor	6 ¼"	4"
				

DRAFT

59	2 Dec 25	Section 4; floor	7"	4"
				
60	2 Dec 25	Section 4; floor	7"	4"
				
61	2 Dec 25	Section 4; floor	7"	4"
				

DRAFT

62	2 Dec 25	Section 4; floor	6 ½"	4"
				
63	2 Dec 25	Section 4; floor	6"	4"
				
64	2 Dec 25	Section 4; floor	6"	4"
				

DRAFT

65	2 Dec 25	Section 4; floor	6"	4"
				
66	2 Dec 25	Section 4; floor	6"	4"
				
67	2 Dec 25	Section 4; floor	6"	4"
				

DRAFT

68	2 Dec 25	Section 4; floor	9 ½"	4"
				
69	2 Dec 25	Section 4; floor	6 ¾"	4"
				
70	2 Dec 25	Section 4; floor	10"	4"
				

DRAFT

71	2 Dec 25	Section 4; floor	6 ¼"	4"
				
72	2 Dec 25	Section 4; floor	7"	4"
				
73	2 Dec 25	Section 4; floor	5 ½"	4"
				

DRAFT

74	2 Dec 25	Section 4; floor	9 ½"	4"
				
75	2 Dec 25	Section 4; floor	6"	4"
				
76	2 Dec 25	Section 4; floor	6 ¼"	4"
				

DRAFT

77	2 Dec 25	Section 4; floor – adjacent to south crack	6 ½"	4"
				
78	3 Dec 25	Section 4; floor Possible fill material below	5 ½"	4"
				
79	3 Dec 25	Section 4; floor	5 ½"	4"
				

80	3 Dec 25	Section 4; floor	6 ¼"	4"
				
81	3 Dec 25	Section 4; floor	6 ½"	4"
				
82	3 Dec 25	Section 4; floor	5 ½"	4"
				

DRAFT

83	3 Dec 25	Section 4; floor @ crack in floor	6 ½"	4"
				
1N	26 Nov 25	Section 1; north wall	6" - 8"	4"
				
2N	26 Nov 25	Section 1; north wall	9" -10"	4"
				

DRAFT

3N	26 Nov 25	Section 1; north wall	5"	4"
				
4N	26 Nov 25	Section 1; north wall	6 1/2"	4"
				
1S	26 Nov 25	Section 1; south wall	6 1/2"	4"
				

DRAFT

2S	26 Nov 25	Section 1; south wall	6"	4"
				
3S	26 Nov 25	Section 1; south wall	8"	4"
				
4S	26 Nov 25	Section 1; south wall	6"	4"
				

DRAFT

5N	28 Nov 25	Section 2; north wall	5"	4"
				
6N	28 Nov 25	Section 2; north wall	7"	4"
				
7N	28 Nov 25	Section 2; north wall	6 1/2"	4"
				

DRAFT

8N	28 Nov 25	Section 2; north wall	7"	4"
				
5S	29 Nov 25	Section 2; south wall	6"	4"
				
6S	29 Nov 25	Section 2; south wall	Could not be retrieved	4"
				

DRAFT

7S	29 Nov 25	Section 2; south wall	14 ½"	4"
				
8S	29 Nov 25	Section 2; south wall	8"	4"
				
9N	1 Dec 25	Section 3; north wall	7"	4"
				

DRAFT

10N	1 Dec 25	Section 3; north wall	8"	4"
				
11N	1 Dec 25	Section 3; north wall	12 1/2"	4"
				
12N	1 Dec 25	Section 3; north wall	5 1/2"	4"
				

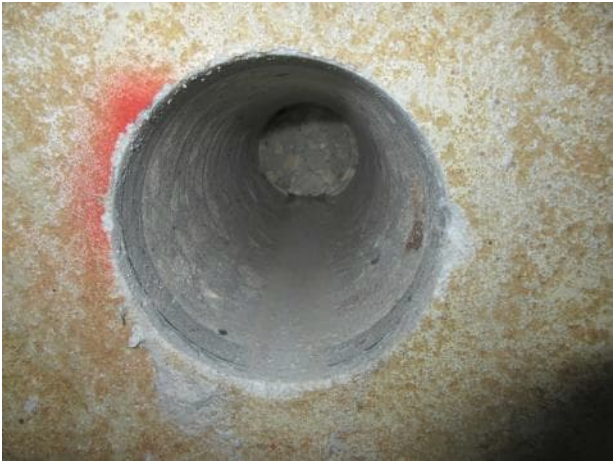
DRAFT

9S	1 Dec 25	Section 3; south wall Large void behind wall	6" – 8"	4"
				
10S	1 Dec 25	Section 3; south wall	14"	4"
				
11S	1 Dec 25	Section 3; south wall	6"	4"
				

DRAFT

12S	1 Dec 25	Section 3; south wall	5 ¼"	4"
				
13N	4 Dec 25	Section 4; north wall	7"	4"
				
14N	4 Dec 25	Section 4; north wall	8 ¼"	4"
				

DRAFT

15N	4 Dec 25	Section 4; north wall	7"	4"
				
16N	4 Dec 25	Section 4; north wall	5 1/2" – 7"	4"
				
17N	4 Dec 25	Section 4; north wall Coating delaminated	6"	4"
				

18N	4 Dec 25	Section 4; north wall	9 ½"	4"
				
19N	4 Dec 25	Section 4; north wall	6 ½"	4"
				
13S	3 Dec 25	Section 4; south wall	7 ¼"	4"
				

DRAFT

14S	4 Dec 25	Section 4; south wall	7 ½"	4"
				
15S	3 Dec 25	Section 4; south wall	8 ½"	4"
				
16S	3 Dec 25	Section 4; south wall Small void behind	14 ½"	4"
				

DRAFT

17S	3 Dec 25	Section 4; south wall	11 ¼"	4"
				
18S	3 Dec 25	Section 4; south wall	8"	4"
				
19S	3 Dec 25	Section 4; north wall	7 ¼"	4"
				

DRAFT